GENERAL NOTES

SCOPE OF WORK

1. The construction work for the project is defined in Drawings S1 through S12, and in the printed Project Manual entitled Bronco Stadium Restoration & Waterproofing Project.

2. This project includes general concrete repairs, expansion joint work, drainage work, sealant work, membrane installation, and other miscellaneous repairs to the East, West, Southeast, and Southwest Balconies. Localized concrete and membrane repairs in the West Balcony and Lower Stadium, expansion joint replacement in the West Balcony, and membrane installation in the Southeast Lower Stands are included as alternates.

3. For more information on the work scope, consult Sheet S-3 and the project specifications, Section 01800 - Expanded Summary of Work.

GENERAL REQUIREMENTS

1. Dimensions and elevations of the existing construction are based on the original design drawings for the structure. Copies of these drawing are available from the Owner upon request.

2. Actual dimensions, elevations, and existing conditions may vary from the original drawings. The Contractor shall field verify the existing dimensions, elevations, and existing conditions prior to starting the work or ordering material. Dimensions of the new construction shall be adjusted as required to fit the as-built, existing conditions.

3. Contractor shall be responsible for protecting and/or removing any wood platforms, exposed conduit, and other existing items adjacent to the balcony work. In the case of removal, the Contractor shall reinstall the above-mentioned items at no extra cost.

4. During all stages of construction, protect stadium elements that are to remain. Any damage caused to the existing building elements which are to remain shall be the sole responsibility of the Contractor and shall be repaired to the satisfaction of the Owner at the Contractor's expense.

5. Riser heights in the balcony may vary per aisle location. Verify riser heights at each row before ordering or installing expansion joint seals, gutters, or other items that may be affected.

6. Normal working hours for the Contractor shall be per the Specification. If circumstances warrant, the Contractor may request permission for extended hours or weekend hours.

7. The Contractor shall develop and keep up-to-date a work plan and schedule for all repair activities so the University can be apprised and plan accordingly.

8. Coordinate any relocation or protection of stadium/building contents with the University prior to and during the repair work.

9. Any construction equipment or materials that are left in or around the stadium during stadium events must be minimized and coordinated with the University to minimize disruption to stadium operations. Barricades, enclosures, and signage shall be used to protect the public and to prevent public access to any work areas during stadium events.

10. During events at the stadium while work is underway (specifically Spring Football, Track Meets, and High-School Football Games), the Contractor shall, at a minimum:

   A. Cease all construction activity the day of the event, and coordinate with the University for reduced construction activities for the few days prior to and after the events.

   B. Schedule and complete work so that each gate is available for use at each scheduled stadium event.

   C. Have the visible portions of the work areas clean and neat prior to and during the event.

   D. Remove from the site or relocate construction equipment and materials that interfere with the stadium operations to areas designated by the University.

   E. Take measures necessary, in cooperation with the University, to prevent public access to the work or storage areas during the events using barricades, enclosures, signage, etc.

11. The Contractor shall provide warning signs, barriers or barricades, as required and otherwise necessary to separate the work area from the public area in the stadium, to prevent pedestrians and traffic from entering the work areas, and to provide for safe and orderly flow of pedestrian and vehicle traffic through the stadium during the repair work.

12. The Contractor shall be responsible for keeping all stadium areas used by the Contractor, subcontractors, and suppliers clean and free of all dirt, mud, and other construction debris, and will be required to clean them as is necessary in order to maintain them in a safe condition. The Contractor shall be especially responsive to requests from the University, or any official of the Owner, to cooperate throughout the project duration.

13. All materials shall be delivered to the job site in manufacturer's sealed packaging and stored in an enclosed shelter providing protection from damage and exposure to the elements. Damaged or deteriorated materials shall be removed from the premises.

SHORING

1. Contractor shall submit shoring design and calculations performed by an Idaho Licensed Engineer prior to shoring operations.

DEMOLITION

1. The extent of the demolition work is as shown on and reasonably inferable from the drawings.

2. Equipment to be used for concrete demolition work on concrete work that shall remain, shall be limited to chipping hammers with a total weight not to exceed 15 lbs. Larger demolition equipment shall be formally approved by the Owner and engineer where no detrimental effect to the structure can be foreseen.

3. Provide for off-site disposal of all demolished material.

4. Perform demolition work during hours designated by Owner.

CONCRETE

1. Reinforcing bars shall conform to the requirements of ASTM 615, Grade 60.

2. Reinforcing bar shop drawings shall be submitted for all new concrete construction.

3. Reinforcing bars designated (E) shall be epoxy-coated. Damage to the epoxy coating during handling, placement, etc., shall be repaired with a compatible epoxy.

4. All accessories, including bolsters, chairs, tie wire, etc., used to tie or support the epoxy-coated bars shall be epoxy-coated.

5. All reinforcement bar laps, splices, and other details shall be in conformance with ACI 318, but in no case shall laps and splices be less than 36 bar diameters, unless noted otherwise. Splicing of epoxy-coated bars shall have the lap length increased by 15 percent. Locations of laps and splices shall be staggered to offset top and bottom splices.

6. All welded wire fabric shall conform to ASTM A185 and shall be of wire gage and mesh size as shown on these drawings. Flat sheet mesh shall be used rather than rolled mesh to control cover and bending.

7. All welded wire fabric reinforcement shall have a minimum lap of one (1) full mesh panel (unless noted otherwise) and be tied securely at all side and end laps. The mesh shall be tied two (2) times in each panel.

8. All cast-in-place concrete shall be normal weight, stone concrete (150 psf) with a minimum 28-day compressive strength of 5,000 psi. The cement content shall be at least 5-3/4 bags per cubic yard. Water-cement ratio shall be such as to produce a slump of 3" ± 1" without exceeding a water-cement ratio of 0.45. The in-place air content shall be 6% ± 1%

9. Materials for use in cast-in-place concrete shall conform to the following:

   A. Portland cement: ASTM C150, Type 1, color gray, Type 3 cement is not permitted.

   B. Air-entraining admixtures: ASTM C260

   C. Chemical admixtures: ASTM C494

   D. Aggregates: ASTM C33

   E. Water: Potable

   F. Flyash: Class F

10. All concrete shall contain an approved air-entraining admixture. No calcium chlorides shall be introduced into any concrete.

11. All concrete reinforcement shall be detailed, fabricated, labeled, supported and spaced in the forms and secured in place in accordance with the procedures and requirements of the Building Code Requirements for Reinforced Concrete (ACI 318) and the Manual of Standard Practice for Detailing Reinforced Concrete Structures (ACI 315).

12. All cast-in-place concrete shall be tested during placement and cylinder samples taken from compressive strength testing, as described in the specifications. At the Engineer's discretion, the air content may be measured both before and after pumping to ensure the specified concrete properties are met.

13. All hot and cold weather concrete placement shall be performed according to ACI 305 and ACI 306, respectively.

14. The Contractor shall notify the engineer at least 48 hours before any scheduled concrete pour to permit inspection of the formwork and reinforcing bar placement.

15. When the temperature is above 80°F, the Contractor shall provide, when required by the Owner or Engineer, an approved admixture for the concrete for retarding the initial set.

16. Epoxy-grouted reinforcing bars shall not be bent after being embedded in hardened, sound concrete unless permitted by the Engineer.

17. Design and construction of formwork shall be the responsibility of the Contractor and shall be performed in accordance with ACI 347.

18. Construction joints designated on the drawings will be filled with a 2-component polyurethane sealant such as Sonneborn NP-2 or an approved equal. Color shall match the substrate.

19. Priming grout used in pumping operations shall be discarded and not used in the concrete placement area.

CONCRETE REPAIR

1. Remove patch mortar or concrete in areas as shown on the drawings. The shape of each removal area shall not be irregular. Angles between adjacent saw cuts around the perimeter of the patch shall not be less than 90 degrees, in any case. Edges shall be saw-cut vertically to the depth of 3/4 in. The depth of sawcut shall be adjusted to avoid cutting existing reinforcing bars or prestressing strand

2. During the concrete removal (chipping) process, care shall be exercised to avoid cracking of the underlying or adjacent sound concrete.

3. During the removal of concrete, if more than half (50 percent) of a reinforcing bar or strand circumference is exposed, or if the bar is not firmly bonded to the surrounding concrete, then the remaining concrete around the bar shall be removed. The clearance between the bar and the concrete shall be a minimum of 3/4 in., measured radially from the surface of the bar.

4. Existing reinforcing shall be cleaned by sandblasting to remove all rust and adhered concrete. After cleaning, all exposed reinforcement shall be carefully evaluated to determine if replacement or additional reinforcement bars are required.

5. Sandblasted concrete surfaces shall be protected from contamination and exposure to the elements prior to concrete casting. If in the opinion of the Architect/Engineer the surface becomes contaminated, the concrete surface shall be recleaned.

6. Do not remove or cut existing embedded reinforcing steel, strand, or mesh, unless specifically designated on the drawings. Existing, embedded steel should be reused, and if the steel has lost significant cross-sectional area, supplemental reinforcing steel shall be installed. Bring severe deterioration conditions to the attention of the Engineer.

7. Segments of existing reinforcing with more then ½ in. cover from the original concrete surface which were partially or fully exposed in concrete removal operations shall receive one coat of epoxy which fully covers the bar. A touch-up coat shall be applied if visible pinholes or holidays remain after the first coat. The dry film thickness of the coating shall be approximately 10 to 12 mils. A second coat of epoxy shall be applied to existing exposed bars with 1/2 in. or less of cover from the original concrete surface.

8. Existing concrete surfaces below the reinforcing shall be protected from epoxy spillage. It is recommended that heavy paper, cardboard, or visqueen be installed below the bars during the coating process. Epoxy spilling on existing concrete surfaces shall be removed by additional chipping or another approved method.

EPOXY-GROUTED DOWELS

1. All drilled-in reinforcing bar dowels and stainless steel anchors shall be set with an approved gel epoxy resin, except that a proprietary adhesive system can be used as approved by the Engineer.

2. All dowels shall be embedded 4 in. into the existing concrete, unless noted otherwise.

3. Following drilling, anchor holes shall be thoroughly cleaned with oil-free and moisture-free compressed air, using a nozzle extended to the bottom of the hole, and supplemented with a brush or other tool as necessary to remove all dust and loose material. Core-drilled holes shall be flushed with clean water immediately following drilling, blown out with compressed air, and permitted to dry. Refer to the Specification for written procedure requirements.

4. Unless otherwise indicated, adhesive or grout shall be place through a tube or gun extension, beginning at the maximum depth of the hole and withdrawn as adhesive is pumped, followed by insertion of the anchor to the specified depth. Spaces around anchors at the surface shall be sealed at vertical and overhead locations where necessary to prevent escape of the adhesive during curing.

CONCRETE EXPANSION ANCHORS

1. Concrete expansion anchors to replace existing, damaged stud bolts used to attach the seating bracket to the concrete shall be 3/8 or 5/8 in. diameter (field verify) stainless steel concrete anchors. The anchors shall be one of the following:

   A. Hilti Kwik-Bolt II Anchor (Stainless Steel), manufactured by Hilti Fastening Systems, Tulsa, OK.

   B. Trubolt Wedge (Stainless Steel), manufactured by ITW/Red Head, Michigan City, IN.

   C. Wedge-All Wedge Anchor (Stainless Steel), manufactured by Simpson Anchor Systems, Dublin, CA.

   D. An approved equal with mill certifications that can sufficiently document the stanless steel physical and material properties.

2. Nuts, washers, and other hardware used with the expansion anchors shall have an alloy designation that matches the anchor alloy (i.e., ANSI Type 304 or 316 for stainless). Replacement hardware for the existing galvanized asemblies shall be hot-dip galvanized. Electroplate galvanizing is not acceptable. Dissimilar metal assemblies shall be separated by nylon, EPDM, or other approved non-metallic washers.

3. The Contractor shall provide all equipment required to install the expansion anchor including, but not limited to, drills, setting tools, clean-out brushes, blow out bulbs, wrenches, etc.

EXPANSION AND ISOLATION JOINTS

1. The new seal in the isolation joints and expansion joint front aisle / vomitories shall be a rapid cure, two-component silicone seal. The color shall be gray to match the substrate. The isolation joint seal system shall be one of the following:

   A. Wabo SiliconeSeal Joint System, as manufactured by Watson-Bowman & Acme Corp., Amherst, New York.

   B. Dow Corning NS Parking Structure Sealant (Non-Sag) and / or SL Parking Structure Sealant (Self-Leveling), as manufactured by Dow Corning, Midland, Michigan.

   C. An approved equal.

2. The new seal in the expansion joint between the tread and risers shall be:

   A. "FW" Series of Jeene Joint, as manufactured by Watson-Bowman & Acme Corp., Amherst, New York.

   B. An approved equal.

3. Materials and installation shall be in accordance with manufacturer's printed installation instructions and relevant specifications.

4. The joint seal manufacturer's representative is to be present at start-up and shall periodically visit site to supervise the proper use and installation of the product.

5. Contractor is to provide access for inspection of joints from above and below after installation. Improper joint seal size, improper placement, and incomplete adhesive application shall be cause of complete removal and replacement at no additional cost in the defective area.

MEMBRANE WATERPROOFING

1. The waterproofing membrane shall be one of the systems specified in the project manual. Materials shall be installed by a manufacturer's approved applicator in strict accordance with the manufacturer's written specifications. All procedures for installation shall comply with recommendations of manufacturer of products being used.

2. The Contractor shall prepare and install a membrane system on one 12 ft x 12 ft stands area selected by the Architect/Engineer, at the start of the work to serve as a field sample. This field sample shall represent all conditions of the finished work. The approved field sample shall be used as the standard to which all remaining membrane waterproofing systems are installed.

3. The standard waterproofing system shall be a three-coat system with the following minimum characteristics:

   A. Primer and base coat with minimum thickness of 25 mils (0.025 in.)

   B. Wear coat with minimum thickness of 30 mils (0.030 in.)

   C. Top coat (UV protection) with minimum thickness of 7 mils (.007 in.)

4. The standard waterproofing system shall have a thicker wear course in all designated aisles and stairs. A minimum thickness of 35 mils shall be used in the aisles and stairs.

5. Paint row numbers, section numbers, and vomitory signage on the membrane surface in accordance with the Owner's instructions. All painting shall match the painted signage provided in the Stadium.

6. Before coating work is commenced, the vertical and horizontal concrete surfaces shall be thoroughly cleaned to remove laitance, loose material on the surface, grease, oil and other contaminants which will affect bond of the membrane. Abrasively prepare by shotblasting or sandblasting all existing horizontal and vertical surfaces and all new horizontal vertical and patch surfaces to receive the membrane. Furthermore, sandblast areas not accessible to shotblast. Remove all surface contaminants. Remove any existing membranes or sealants on the slab surface. Use a concentrated, dry air blast in all cracks to remove dust, sand, shotblast pellets, etc. Surfaces shall be left broom, vacuum, or airblasted clean.

7. All concrete must have full 28-day cure period prior to coating. Verify that curing methods used for concrete are compatible with coating system.

8. Surfaces to receive a membrane are to be even. Protrusions and bumps are to be ground flat and with a similar texture as the surrounding area. Pits, scaled areas, depressions or traction grooves in the surface which will inhibit a uniform membrane application are to be filled with an epoxy mortar, or a similar material that is compatible with the membrane system.

9. Rout or sawcut cracks exceeding 1/16 in. in width and fill with sealant, per Section 07951. Sealant shall be tested for compatibility with membrane and approved in writing for use by both the sealant and membrane manufacturers.

ACRYLIC & SILANE SEALERS

1. Designated concrete surfaces shall be cleaned and coated with a silane surface sealer per Sheet S-8. The sealer shall be isobutyl trimethoxy silane in a 40% solids solution of anhydrous isopropyl alcohol applied in two coats. Surfaces to receive silane include the vehicular ramps, and other Engineer designated surfaces. The silane sealer shall be one of the following:

   A. SIL-ACT ATS-42, by Advanced Chemical Technologies

   B. Dynasylan BH-N, from Degussa-Huls

   C. Hydrozo Silane 40, by ChemRex

   D. An approved equal

2. Designated concrete wall surfaces shall be cleaned and coated with a water-dispersed, acrylic protective coating. Apply acrylic coating with two (2) coats, minimum to achieve a dry film thickness (DFT) of 4 to 6 mils. The acrylic protective coating shall be one of the following:

   A. Sikagard 670 W, from Sika Corporation

   B. An appoved equal.

3. At the cracks in the vehicular ramp slabs and other locations identified on the drawings, gravity feed an epoxy into the noticeable cracks allowing potential moisture migration to the concrete interior. See the specifications for further information and products.

DRAINS AND DRAIN COVERS

1. Drains: New floor drains, grates, and replacement grates for broken covers in the front row shall match the existing in size and type. Acceptable manufacturers include:

   A. Zurn Industries, Inc., Erie, Pennsylvania (814) 455-0921

   B. J.R. Smith, Montgomery, Alabama, (334) 277-8520. Present drains used in the East, Southeast, and Southwest balconies.

   C. Josam, Michigan City, Indiana, (800) - 36JOSAM. Present drains used in the West balcony.

   D. An approved equal.

2. Floor grates shall be flat and rated for heavy duty traffic. The grates shall include a maximum number of openings. Submit product literature for drains and grates used in each location.

3. Grates: New floor drains covers for the Row 1 or A drains shall be new dome grate covers to fit the existing size and type of drain present. Present drains on site include:

   A. East, Southeast, and Southwest balconies - J.R. Smith

   B. West balcony - Josam.

4. Connect piping for all new drainage fixtures to the existing drainage pipes. Adhere to all applicable Idaho Plumbing Codes in their installation.

SEALANTS

1. Concrete crack sealant shall be Sonneborn NP1 or NP2, or an approved equal. Sealant shall be compatible with the membrane. Color of the sealant on exposed surfaces shall match the substrate.

2. All materials used in combination, i.e., sealants with backer rods, or sealants with primers shall be in conformance with Sealant manufacturer printed instructions. Sealant manufacturer must be consulted prior to application and is responsible for co-signing the special warranty (see Section 01740).

3. All surfaces to receive the joint sealants shall be examined by the Contractor. Any surfaces which are found to be unsuitable for installation of the joint sealants shall be brought to the attention of the Architect/Engineer for resolution. Application or installation of the material constitutes acceptance of the surface of the substrate.

4. Test applications shall be made at the beginning of the joint sealant work, in all types of prepared joints, by the Contractor, to determine if preparation steps have been adequate for optimum sealant adhesion. These test applications shall be approved by the Architect/Engineer prior to the start of work.

5. Remove all existing sealants from the areas to be resealed. Care shall be used in the removal of sealants so as not to damage existing construction intended to remain.

6. All surfaces to receive sealants shall be clean, dry, free of any loose materials, dirt, dust, laitance, rust, oil, frost, and other contaminants.

7. Existing concrete cracks shall be routed out with a power grinder, saw or other equipment capable of creating the required recess for the sealant, and blast cleaned with oil free compressed air to remove the dust of cutting. Cleaning solvents shall not be used on the concrete.

8. Install bond breakers and backer rods where shown on the drawings and in locations and of the type recommended by the sealant manufacturer to prevent bond of sealant to surfaces where such bond might impair the performance of the sealant.

STRUCTURAL STEEL

1. Structural steel material shall conform to ASTM A36 (Fy = 36 ksi, min.). Welding electrodes shall conform to E70XX.

HANDRAIL

1. Any new handrail assemblies shall be fabricated from round aluminum pipe section to match other railing fabrications in the stadium.

2. The Contractor shall submit fabrication shop drawings of the replacement handrails for approval.

3. All new or repaired handrail posts shall be provided with a half-round weep hole at the base (radius = 0.5 in.). The holes shall be drilled and ground smooth prior to installation. New post base grout shall stop short of the weep hole and not cover the opening.

4. Post bases of aluminum handrail shall be coated with a heavy bituminuous paint on the interior and exterior surfaces to be in contact with the new grout pocket material.

TEFLON SLIDE BEARINGS

1. Stainless steel plate shall conform to ASTM A240/A240M Type 304.

2. The Teflon (TFE) sheet shall conform to the requirements of the AASHTO Standard Specifications for Highway Bridges

3. The stainless steel plate in contact with the TFE sheet shall be polished to a Type 2B or 3 or smoother finish to provide a surface finish of 0.01 mils root mean square or less.

4. The lower assembly for expansion bearings shall consist of a ROF pad with a 94 mil thick, low friction, virgin, unfilled, polytetrafluroethylene (TFE / PTFE or Teflon) sheet conforming to ASTM D 4894 bonded to the upper surface. Acceptable products and manufacturers include:

   A. Dynalon Slide Bearings on Masticord, by JVI Inc., Lincolnwood, Illinois

   B. PFTE on Fiberlast Pad, by Voss Engineering, Lincolnwood, Illinois

   C. An approved equal.

5. Shop drawings of the bearing assemblies shall be submitted to the Engineer. The bearing assemblies shall be furnished as a complete unit from one manufacturing source.

6. During the cleaning and painting, the stainless steel and TFE sheet sliding surfaces and the elastomer shall be protected from abrasion and paint.

7. Bonding of TFE sheets shall be done as noted on the plans and in the Specification. No rubber flash will be permitted on the edges of TFE bearing surfaces. All burrs or raised edges along the perimeter of the TFE surface shall be removed before shipment.

8. Headed stud shall be 3/4 in. diameter Nelson S3L studs x 6 3/16 in. as manufactured by the Nelson Stud Welding, Elyria, Ohio, or an approved equal.

9. Carbon steel plate shall conform to ASTM A36 (Fy = 36 ksi, min.)

10. All exposed plain steel surfaces shall be blast cleaned to a near white finish and zinc primed with a minimum thickness of 4 mils. Flame cut edges of the sole plates shall be ground to reduce hardness and facilitate blast cleaning. All corners of plates shall be rounded to a 1/16 inch radius.

SHEET METAL FLASHING

1. Painted aluminum sheet metal shall be manufactured by the Peterson Aluminum Corporation, or an approved equal, of the size, type and gage shown on the drawings.

2. Termination bars contacting and attaching the stanless sheet metal shall be painted aluminum as dimensioned on the drawings and attached on 1 ft maximum centers, unless noted otherwise.

3. Submit shop drawings of all sheet metal work showing details of fabrication, installation anchorage, and interface of the work of this Section and the work of adjacent trades.

4. Install new sheet metal fabrications and accessories as shown on the Drawings, with all sharp edges eliminated. Install new sheet metal fabrications true to lines and levels.

REFERENCED STANDARDS (Latest Edition/Revision Shall Apply)

1. American Concrete Institute (ACI)

   A. Building Code Requirements for Structural Concrete (ACI 318)

   B. Manual of Standard Practice for Detailing Reinforced Concrete Structures (ACI 315)

   C. Specifications for Structural Concrete for Buildings (ACI 301)

   D. Recommended Practice for Concrete Formwork (ACI 347)

2. International Code Council (ICC)

   A. International Building Code (IBC)

3. American Institute for Steel Construction (AISC)

   A. Manual of Steel Construction

   B. Detailing for Steel Construction

4. American Welding Society (AWS)

   A. Structural Welding Code - Steel (AWS D1.1)

ABBREVIATIONS

$\sim$	Approximately
$\pm$	Either Direction
ALT	Alternate
BM	Beam
BRG	Bearing
CC	Center-to-Center
CIP	Cast-In-Place
℄	Centerline
COL	Column
CP	Complete Penetration
CONC	Concrete
CONT	Continuous
CONTR	Contractor
DIA	Diameter
ϕ	Diameter
DBL	Double
E	East
(E)	Epoxy-Coated Reinforcement
EA	Each
EF	Each Face
ELEV	Elevation
ES	Each Side
EW	Each Way
EQ	Equal (spacing)
EXIST	Existing
FLG	Flange
FS	Far Side
HORIZ	Horizontal
HP	High Point
HS	High Strength
J.O.	Joint Opening
LLV	Long Leg Vertical
LLH	Long Leg Horizontal
LONG	Longitudinal
LONGIT	Longitudinal
LP	Low Point
MAX	Maximum
MIN	Minimum
NIC	Not In Contract
NS	Near Side
NTS	Not To Scale
OC	On Center
OPP	Opposite
OH	Over Head
PL	Plate
REQ'D	Required
SIM	Similar
SE	Southeast
SQ	Square
SS	Stainless Steel
SW	Southwest
T & B	Top and Bottom
T/	Top of
TYP	Typical
UNO	Unless Noted Otherwise
VERT	Vertical
VIF	Verify in Field
W	West
W/	With
W/O	Without
WPT	Working Point

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PROFESSIONAL ENGINEER REGISTERED 8/29/05 STATE OF IDAHO WILLIAM J. NUGENT

LICENSED ARCHITECT AR-1577 8-29-05 ROBERT L. SMITH STATE OF IDAHO

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Date: 29 AUGUST 2005
DPW No: 05209
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FILE NAME:
REV:

SHEET TITLE: GENERAL NOTES & ABBREVIATIONS

DRAWING S-2